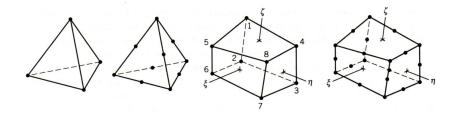
Solid Elements



All images are from R. Cook, et al. Concepts and Applications of Finite Element Analysis, 1996.

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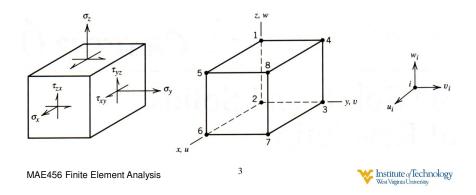


Introduction

- Problems of beam bending, plane stress, plates, etc. may be considered as special cases of 3D solids.
- So why not use 3D solids all the time?

Introduction

- Stress can vary in all three directions
- Nodes are located in 3 space
- Nodes have displacements in 3 directions



Stress-strain relations

• Stress-strain are now related by a 6x6 matrix

$$\begin{cases} \sigma_{x} \\ \sigma_{y} \\ \sigma_{z} \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{cases} = \begin{bmatrix} (1-v)c & vc & vc & 0 & 0 & 0 \\ & (1-v)c & vc & 0 & 0 & 0 \\ & & (1-v)c & 0 & 0 & 0 \\ & & & G & 0 & 0 \\ & & & & G & 0 \\ & & & & G \end{bmatrix} \begin{bmatrix} \varepsilon_{x} \\ \varepsilon_{y} \\ \varepsilon_{z} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{bmatrix}$$
symmetric
$$G = \begin{bmatrix} \sigma_{x} \\ \sigma_{y} \\ \sigma_{z} \\ \sigma_{z} \\ \sigma_{zx} \end{bmatrix}$$

$$c = \frac{E}{(1+v)(1-2v)}$$
 and $G = \frac{E}{2(1+v)}$

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Stress-strain relations

• To get strain from stress, use:

$$\begin{cases} \mathcal{E}_{x} \\ \mathcal{E}_{y} \\ \mathcal{E}_{z} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{cases} = \begin{bmatrix} 1/E & -v/E & -v/E & 0 & 0 & 0 \\ -v/E & 1/E & -v/E & 0 & 0 & 0 \\ -v/E & -v/E & 1/E & 0 & 0 & 0 \\ 0 & 0 & 0 & 1/G & 0 & 0 \\ 0 & 0 & 0 & 0 & 1/G & 0 \\ 0 & 0 & 0 & 0 & 0 & 1/G \end{bmatrix} \begin{bmatrix} \sigma_{x} \\ \sigma_{y} \\ \sigma_{z} \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{bmatrix}$$

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Strain-Displacement Relations

• If strains are small:

$$\varepsilon_{x} = \frac{\partial u}{\partial x} \qquad \gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$$

$$\varepsilon_{y} = \frac{\partial v}{\partial y} \qquad \gamma_{yz} = \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y}$$

$$\varepsilon_{z} = \frac{\partial w}{\partial z} \qquad \gamma_{zx} = \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z}$$

 These are the most general equations. In deriving previous elements we made assumptions about some of these values (e.g., in plane strain we assumed ε_z=0)

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Displacement Interpolation

 Displacements within an element are interpolated from nodal displacements using u=Nd, as before, however there are now interpolations in three directions.

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General Formula for k

 The general, energy-based formula for k is the same as for previous elements, except that there are more terms in the integration due to the bigger matrices.

$$\mathbf{\varepsilon}_{6\times 1} = \mathbf{B}_{6\times 3n} \mathbf{d}_{3n\times 1}$$

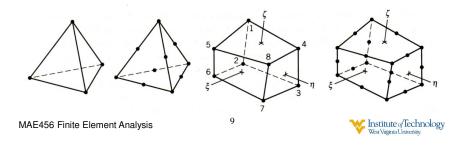
$$\mathbf{k}_{3n\times 3n} = \iiint \mathbf{B}^T \mathbf{E}_{6\times 6} \mathbf{B} \, dx \, dy \, dz$$

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3D Solid Elements

- The 3D solid elements are analogous to planar counterparts:
 - 3-Node Triangle → 4-Node Tetrahedron
 - 6-Node Triangle → 10-Node Tetrahedron
 - 4-Node Quadrilateral → 8-Node Hexahedron
 - 8-Node Quadrilateral → 20-Node Hexahedron



4-Node Tetrahedron

 As with the 3-Node Triangle, it is only accurate when strains are almost constant over the element span.

$$u = \beta_1 + \beta_2 x + \beta_3 y + \beta_4 z$$

$$v = \beta_5 + \beta_6 x + \beta_7 y + \beta_8 z$$

$$w = \beta_9 + \beta_{10} x + \beta_{11} y + \beta_{12} z$$

Trilinear Hexahedron

• Is also called the 8 node "brick" element. $u = \beta_1 + \beta_2 x + \beta_3 y + \beta_4 z + \beta_5 xy + \beta_6 yz + \beta_7 zx + \beta_8 xyz$ $v = \beta_9 + \beta_{10} x + \beta_{11} y + \beta_{12} z + \beta_{13} xy + \beta_{14} yz + \beta_{15} zx + \beta_{16} xyz$ $w = \beta_{17} + \beta_{18} x + \beta_{19} y + \beta_{20} z + \beta_{21} xy + \beta_{22} yz + \beta_{23} zx + \beta_{24} xyz$

• Solving for shape functions:

$$u = \sum_i N_i u_i \qquad v = \sum_i N_i v_i \qquad w = \sum_i N_i w_i$$
$$N_i = \frac{1}{8} (1 \pm \xi) (1 \pm \eta) (1 \pm \zeta)$$

• Isoparametrically:

 $\mathbf{k} = \int_{-1}^{1} \int_{-1}^{1} \mathbf{B}^{T} \mathbf{E} |\mathbf{B}| |\mathbf{J}| d\xi d\eta d\zeta$

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Distributed Loading

 The work equivalent nodal loads for a constant distributed pressure p are as shown.

