# Modeling Errors and Accuracy

And checking the results!

MAE 456 Finite Element Analysis



### **Sources of Error**

Causes of incorrect results:

- 1. Mistakes (e.g., forgetting a load)
- 2. Errors
  - A. Modeling Error (due to simplifying assumptions in mathematical model such as when using beam elements we assume that the cross-sections stay planar and do not change shape)
  - B. Discretization Error (i.e., due to piecewise approximation which can be minimized by using higher order shape functions or smaller elements)
  - C. Numerical Error (due to limited number of significant digits maintained by computer)

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- Common mistakes that will cause a singular **K** matrix (and therefore no results):
  - v = 0.5 in a plain strain, axisymmetric or 3D solid element
  - E = 0 in an element
  - No supports, or insufficient supports
  - Part of the model is a mechanism
  - Large stiffness differences
  - In an element with stress-stiffening, negative stiffening has reduced the stiffness to zero
  - In nonlinear analysis, supports or connections have reached zero stiffness

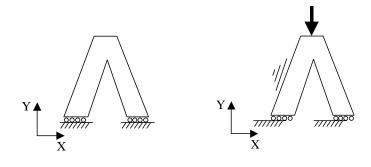
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### **Common Mistakes**

Insufficient supports will allow rigid body motion.



(The stiffness matrix will be singular.)

West Virginia University.

### **Common Mistakes**

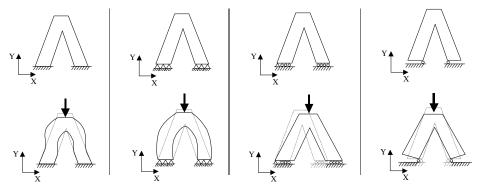
- Mistakes that may go unnoticed:
  - Incorrect element data (e.g., wrong thickness, beam cross-section, cross-section dimension, beam orientation)
  - Supports wrong in location, type or direction
  - Loads wrong in location, type, direction or magnitude
  - Units mix-up
  - A force or mesh defined twice and/or on different duplicated geometry
  - Connections not working as intended (e.g. beam element connected to plane element does not transfer moment)

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## **Common Mistakes**

• Effect of wrong support types:

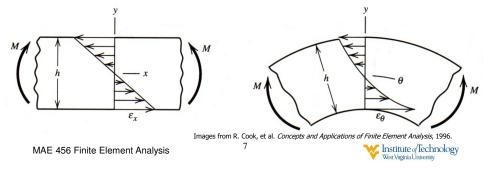


Each of these will result in different displacements, strains and stresses. 6 MAE 456 Finite Element Analysis

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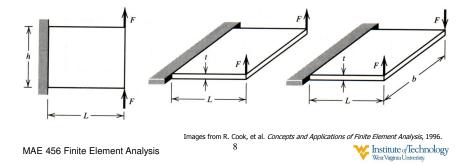
#### 2. A. Modeling Error

- To do a proper FE analysis, the analyst must understand how the structure is *likely* to behave and how elements are *able* to behave.
- E.g., if the analyst knows the displacement varies linearly, 4-node quad. elements will work, but if they vary quadratically, 8-node quad. elements must be used.



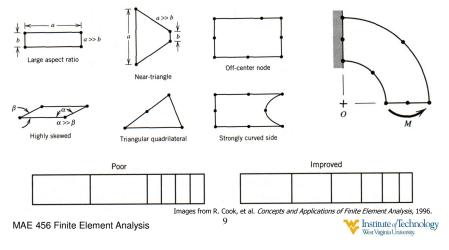
#### **Element Tests**

- Use a *patch* test or *single element* test to determine how an element works under different circumstances.
- Study different states of stress and strain.



### **Element Tests**

• Study the effects of element distortions and changes in element size.

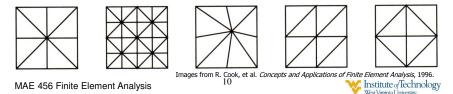


#### **Test Cases**

- · Established test cases from:
  - research literature
  - National Agency for Finite Element Methods and Standards
  - software documentation

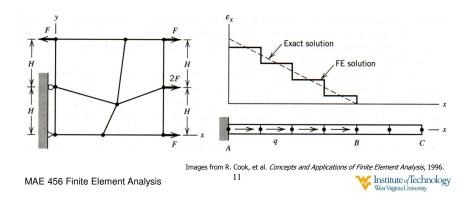
can be used to check the accuracy of elements and models.

• "Pilot studies" can be used to check software capabilities.



#### 2. B. Discretization Error

- If a mesh is repeatedly refined, will the results converge to a solution?
- Yes, if the elements used pass the "patch" test.



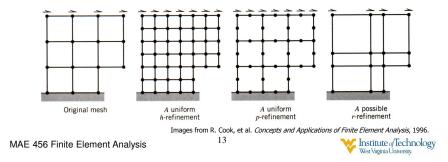
### **Convergence Requirements**

- In a patch test, the FE model must have:
  - A simple arrangement of elements with one internal node
  - Supports sufficient to stop rigid body motion
  - Work equivalent loads consistent with a constant state of stress (and strain)
- To pass the test, the results must exactly represent the correct constant stress (and strain), within numerical error.



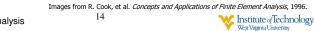
### **Mesh Refinement**

- There are three ways to refine a mesh:
  - 1. *h*-refinement (changing the element size)
  - 2. *p*-refinement (changing to elements with higher order polynomial interpolations)
  - 3. *r*-refinement (moving nodes)



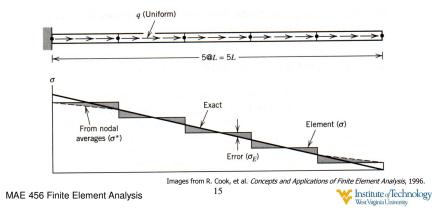
### **Mesh Refinement**

- A combination of these methods can also be used.
- The mesh should be refined until convergence is achieved (i.e., the results change very little from the previous refinement).
- Some software automates the refinement process (adaptive meshing).



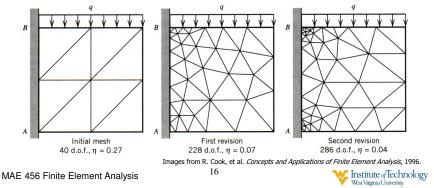
#### **Error Measures**

One approach to error estimation is to assume that the nodal averaged stress ( $\sigma^*$ ) is correct and the error ( $\sigma_E$ ) is given at every point by the difference from the element stress ( $\sigma$ ).



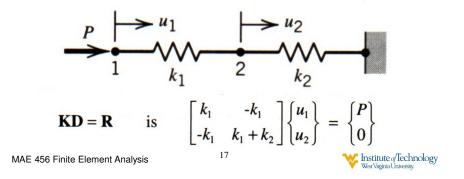
#### **Error Measures and Adaptivity**

 An Automated Adaptive Solution proceeds by refining the mesh, in elements where the error is large, until the maximum error is below some limit.



### 2. C. Numerical Error

- Rounding errors will accumulate (more so in large DOF models)
- Adding very small numbers to big numbers is even more problematic (i.e. in "Stiff Systems")



#### Numerical Error – "Stiff" Systems

• If  $k_1 >> k_2$ ,

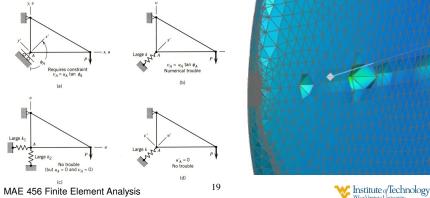
• If 
$$k_2 >> k_1$$
,

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### Susceptibility to III-Conditioning

- Large cross-coupling stiffness coefficients will cause problems
- Having membrane stiffness ≫ bending stiffness will cause problems.



### 3. Checking the Model (before solving)

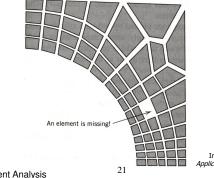
#### · Checking done automatically by software

- Model has mesh and boundary conditions are applied.
- All mesh and boundary condition properties have been provided.
- Element aspect ratios and corner angles too small or too large.
- Element is too warped.
- Poisson's ratio too large.
- Curved shell element spans too great an arc.
- Specific checks that can be requested
  - Coincident nodes (Are they supposed to be one node?)



### Checking the Model (before solving)

- · Checking done by Analyst
  - Everything meshed properly?
  - All required loads/support conditions applied?
  - Double-check material/shell/beam properties

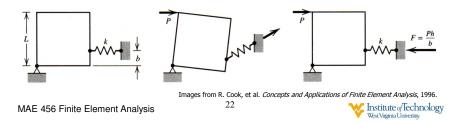


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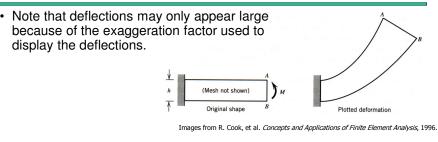


### Checking the Results (after solving)

- · Results should be checked so that:
  - Deflections obey intended support conditions.
  - Deflections are symmetric in a symmetric problem.
  - Where a gap closes the parts **do not overlap**.
  - Support reactions agree with static calculations.
  - There are no large displacements that cause force directions to change (use a nonlinear analysis in this case).



## **Checking the Results**



- Stresses should be checked that:
  - Stress contours are normal to planes of symmetry
  - One of the principle stresses should be zero at an unloaded boundary or equal to -p if there is a pressure p loading condition.

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## **Checking the Results**

- When checking stresses note that:
  - Unaveraged stresses should be checked.
  - Confirm that the displayed stress is the one you want to look at (i.e., principle stress vs. shear stress vs. von Mises stress, etc.)
  - Stresses may be in local or global coordinates
  - Stresses may be for the upper, lower or midsurface of beam and shell elements.



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